

Precast Connection

Application Examples

The following document contains user information with regard to Precast Connection Application Examples





Introduction

RBA16TIS -16mm ReidBar™ Steel Threaded Insert



ramsetreid™'s continued dedication to industry best practice has supported our latest iteration of innovative testing on common connections systems. The idea here was to better understand the performance of connections when the precast elements are subjected to the loads and forces you would expect during a seismic event.

This document provides examples of installation and performance details of ramsetreid products for a series of applications, including cantilevered wall to slab foundation

connections, suspended floor to wall connections, and grouted metal duct connections. NZS3101:2006 A3 Compliance information on the couplers and anchorages is set out below.

NZ53101:2006 A3

The ReidBar couplers and anchorages referred to in this document have been tested and found to comply with the clauses below. The tests were conducted utilising Ramset™ Epcon™ C8 as a filler inside the couplers and anchorages. Refer to the ReidBar Steel Components Specification & Installation Guide for further installation information.

Section 8.7.5.2(b)

"Mechanical connections shall:
...... (b) when tested in tension or compression, as appropriate, to the application, exhibit a change in length at a stress of 0.7fy in the bar, measured over the length of the coupler, of less than twice that of an equal length of unspliced bar." 1

Section 8.9.1.3(a) - Couplers Only

"For welded splices or mechanical connections to be used in members that are subjected to seismic forces, such splices shall comply with 8.7.4.1 or 8.7.5.2. In addition to the requirements of 8.7.5.2, mechanical splices and anchorages shall satisfy the cyclic load performance requirements specified by ISO 15835-1 and ISO 15835-2 as follows: (a) When tested in accordance with 5.6.2 of ISO 15835-2, the residual elongations after 4 cycles, u4, shall be less than 0.3 mm, and after 8 cycles u8 shall be less than 0.6 mm." 1

Section 8.6.11.4

"Mechanical anchors for the anchorage of reinforcing steel shall be proven by an appropriate test method to possess resistance to brittle fracture at the service temperatures at which they are intended for use. Where the mechanical anchors and ends of the bars are threaded as the means of achieving the connection between components, and/or the end of the bar is enlarged by cold forging prior to threading, appropriate testing of the processed bar end shall be applied to ensure that the potential for brittle fracture is avoided. Anchors manufactured from cast iron shall not be used." 1

Compliance with Section 8.6.11.4 was demonstrated via. Charpy V Notch Testing to AS1544.2:2003.

Section 8.6.11.1

"For reinforcement complying with AS/NZS 4671, any mechanical device used alone as an anchorage, or used in combination with an embedment length beyond the point of maximum stress in the bar, shall be capable of developing the upper bound breaking strength of the reinforcing bar without damage to the concrete or overall deformation of the anchorage. In addition, when tested with a bar complying with AS/ NZS 4671, the mode of failure of the anchored bar shall be by ductile yielding of the bar, with the bar developing its ultimate tensile strength at a location outside the mechanical anchorage and away from any zone of the bar affected by working (e.g. by cold forging)." 1

Compliance with Section 8.6.11.1 was achieved with the following product configurations:

Section 8.6.11.1 Product Configuration Table:

Product	Nailing Plate Thickness	Concrete Strength	Edge Distance, e (mm)	Spacing, a1 (mm)	Configuration
RB12TIS	8mm	40MPa	120	300	
RBA16TIS	42mm	40MPa	180	350	Edge

Testing Condition: Uncracked Concrete with products in pairs.

Please Note:

This document is intended for use by Professional Structural Engineers, and the examples provided herein do not remove the need for detailed design by these Professionals.

¹ Reference: NZS3101:2006 Amendment 3 2018



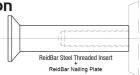
Application Examples Summary Table

Connection Description	Application	ReidBar™ or Reinforcing Components
Application 1: Cantilevered Connection for Precast Panel 150mm thick	Precast Panel wall to slab connection - Single level concrete structure	RB12TIS - 12mm ReidBar Steel Threaded Inserts RB12NP - 12mm ReidBar Nailing Plates RB12 ReidBar Starter Bar D10 Link Bar to drawing
Application 2: Cantilevered Connection for Precast Panel 200mm thick	Precast Panel wall to slab connection - first level for multilevel concrete structure	RBA16TIS - 16mm ReidBar Steel Threaded Inserts RBA16NP - 16mm ReidBar Nailing Plates RBA16 ReidBar Starter Bar R10 Link Bar to drawing
Application 3: Suspended Floor Connection for Precast Panel 200mm thick	Precast Panel wall to floor connection - for multilevel concrete structures	RBA16TIS - 16mm ReidBar Steel Threaded Inserts RBA16NP - 16mm ReidBar Nailing Plates RBA16 ReidBar Starter Bar R10 Link Bar to drawing
Application 4: Grouted Metal Duct Connection for Precast for 150mm, 175mm and 190mm panel thickness	Precast Panel to Precast Panel /Foundation horizontal structural joint	HD12 & Ø50mm Drossbach x 650mm long HD16 & Ø60mm Drossbach x 860mm long HD20 & Ø60mm Drossbach x 1050mm long HD20 & Ø70mm Drossbach x 1050mm long Reinforcing Bar Gr500E Ramset Premier Grout MP
Application 5: Grouted Metal Duct Connection for Precast for 200mm 225mm and 250mm panel thickness	Precast Panel to Precast Panel /Foundation horizontal structural joint	HD16 & Ø60mm Drossbach x 860mm long HD20 & Ø70mm Drossbach x 1100mm long HD25 & Ø70mm Drossbach x 1300mm long HD32 & Ø70mm Drossbach x 1600mm long Reinforcing Bar Gr500E Ramset Premier Grout MP



Application : Cantilevered Connection

I2mm ReidBar™ Steel Threaded Insert (RBI2TIS) Detail for I50mm thick precast panel



Side **Front** bw View View HD10-200 CTS TYPICAL UNO HD12-a1/2 CTS -D10 L**IN**K BAR @ **a**1 CTS RB12-a1 CTS STEEL THREADED INSERT RB12TIS Epcon C8 TYPICAL UNO 42 az R30 TYP a₁ RB12-a1 CTS TOP AND BOTTOM TYPICAL UNO

Installation and performance details (out of plane actions - per anchor)

ReidBar Threaded Insert Size	Anchor Effective Depth, h (mm)	Anchor Spacing				ete Min.	Reduced Characteristic Capacity*			
					Concrete		Gr500E Steel		Concrete	
				Edge distance, e (mm)	Slab thickness, t _{SLAB}	Precast Concrete Panel	oncrete anel ickness,		Tension, ØN _{uc} (kN)**	
		a ₁ (mm)	a ₂ (mm)		(mm)	b _w (mm)		Tension, ØN _{us} (kN)***	Conc. Strength	
									40 MPa	
RB12TIS +		300	300	120	470				44.9##	
8mm thick Nailing Plate & EPCON C8	104	300	300	150	500	150	25.3	42.4	51.8	
		350	300	150	500				54.4	

Please Note:

*Note: Reduced characteristic ultimate steel shear capacity = $\emptyset V_{_{IIS}}$ where \emptyset = 0.65 and $V_{\rm LS}$ = characteristic minimum ultimate steel shear capacity. Concrete Shear capacities can be derived by calculation in accordance with Chapter 17 of NZS3101:Part 1: 2006 A3. **Note: Reduced characteristic ultimate concrete tensile capacity = $\emptyset N$ where $\emptyset = 0.65$ and $N_{uc} =$ Characteristic ultimate concrete tensile capacity.

***Note: Reduced characteristic ultimate steel tensile capacity = ØN, s $\emptyset f_{sv}$ where $\emptyset = 0.75$ and $f_{sv} =$ characteristic yield steel tensile capacity

*Note: Design Tensile Capacity $\emptyset N_{ij} = minimum of \emptyset N_{ij}$ and $\emptyset N_{ij}$ ##Note: This data has been derived through testing at ramsetreid

facility, independently witnessed by Melbourne Testing Services, a NATA accredited laboratory. All other data is derived by calculation in accordance with Chapter 17 of NZS3101:Part 1:2006 A3.

General Note: All data is based on uncracked concrete. For cracked concrete performance, multiply $\emptyset N_{\text{uc}} \times 0.79$



Application 2: Cantilevered Connection

Front

View

I6mm ReidBar™ Steel Threaded Insert (RBAI6TIS) Detail for 200mm thick precast panel



Side bw View EACH FACE HD16-300 CTS EACH FACE RBA16-a1 CTS STEEL THREADED INSERT 35 COVER RBA16TIS Epcon C8 TYPICAL UNO 22 az tslab Φ RBA16-a1 CTS TOP AND BOTTOM TYPICAL UNO

a₁

Installation and performance details (out of plane actions - per anchor)

		Anchor Spacing				Min. Precast	Reduced Characteristic Capacity*		
	Anchor Effective Depth, h (mm)				Concrete		Gr500E Steel		Concrete
ReidBar Threaded Insert Size				Edge distance, e (mm)	Slab thickness, t _{SLAB} (mm)	Concrete Panel thickness,			Tension, ØN _{uc} (kN)**
			a ₂ (mm)			b _w (mm)	Shear, Tension, $\emptyset V_{us}$ (kN)* $\emptyset N_{us}$ (kN)***		Conc. Strength
									40 MPa
RBA16TIS + 8mm thick Nailing Plate & EPCON C8	121	350	300	180	530	200	45.1	75.4	73.5##

Please Note:

*Note: Reduced characteristic ultimate steel shear capacity = $\emptyset V_{_{US}}$ where $\emptyset = 0.65$ and $V_{_{LS}} =$ characteristic minimum ultimate steel shear capacity. Concrete Shear capacities can be derived by calculation in accordance with Chapter 17 of NZS3101:Part 1: 2006 A3.

**Note: Reduced characteristic ultimate concrete tensile capacity = ØN where \emptyset = 0.65 and N_{uc} = Characteristic ultimate concrete tensile capacity.

***Note: Reduced characteristic ultimate steel tensile capacity = $\emptyset N_{_{\rm IS}}$ = $\emptyset f_{sy}$ where \emptyset = 0.75 and f_{sy} = characteristic yield steel tensile capacity

*Note: Design Tensile Capacity $\emptyset N_{ur} = minimum of \emptyset N_{uc}$ and $\emptyset N_{us}$

##Note: This data has been derived through testing at ramsetreid facility, independently witnessed by Melbourne Testing Services, a NATA accredited laboratory.

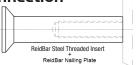
General Note: All data is based on uncracked concrete. For cracked concrete performance, multiply $\emptyset N_{\text{\tiny LIC}} \times 0.63$



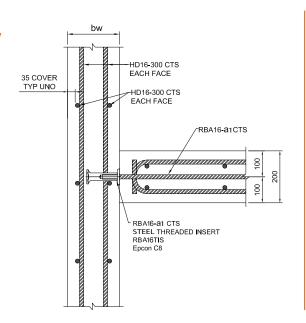
Application 3:

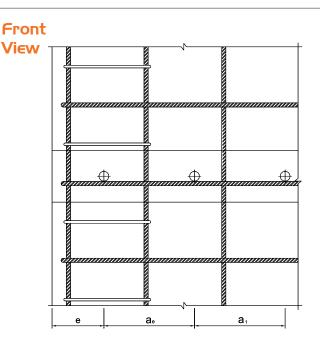
Suspended Floor Connection

I6mm ReidBar™ Steel Threaded Insert (RBAI6TIS) Detail for 200mm thick precast panel



Side View





Installation and performance details (out of plane actions - per anchor)

		Anchor Spacing				Reduced Characteristic Capacity#			
	Anchor Effective Depth, h (mm)				Min. Precast	Gr500	DE Steel	Concrete	
ReidBar Threaded Insert Size		a _e (mm)	a _₁ (mm)	distance, e (mm)	Concrete Panel thickness,		Tension, ØN _{us} (kN)***	Tension, ØN _{uc} (kN)**	
					b _w (mm)	Shear, ØV _{us} (kN)*		Conc. Strength	
								40 MPa	
RBA16TIS + 8mm thick Nailing Plate & EPCON C8	121	350	350	180	200	45.1	75.4	73.5##	

Please Note:

*Note: Reduced characteristic ultimate steel shear capacity = $\emptyset V_{_{IIS}}$ where $\emptyset = 0.65$ and $V_{ij} =$ characteristic minimum ultimate steel shear capacity. Concrete Shear capacities can be derived by calculation in accordance with Chapter 17 of NZS3101:Part 1: 2006 A3.

**Note: Reduced characteristic ultimate concrete tensile capacity = $\emptyset N_{ii}$ where \emptyset = 0.65 and N_{uc} = Characteristic ultimate concrete tensile capacity. cracked concrete performance, multiply $\emptyset N_{uc} \times 0.63$

***Note: Reduced characteristic ultimate steel tensile capacity = ØN, s $\emptyset f_{sv}$ where $\emptyset = 0.75$ and $f_{sv} =$ characteristic yield steel tensile capacity

*Note: Design Tensile Capacity $\emptyset N_{iir} = minimum of \emptyset N_{iir}$ and $\emptyset N_{iir}$

##Note: This data has been derived through testing at ramsetreid facility, independently witnessed by Melbourne Testing Services, a NATA accredited laboratory.

General Note: All data is based on uncracked concrete. For

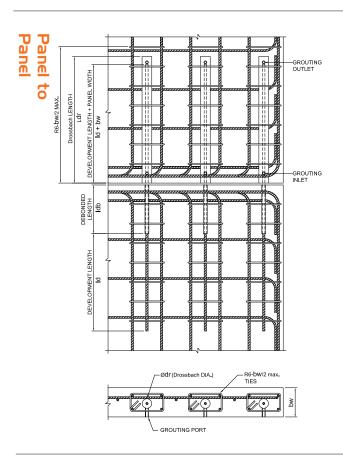


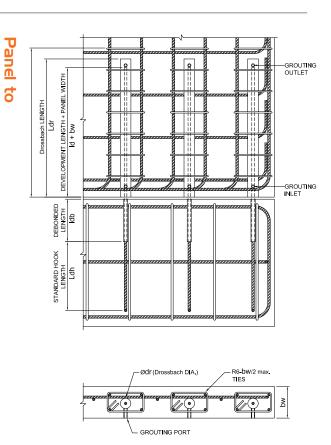
Application 4:

Grouted Metal Duct Connection

Drossbach Detail for ISOmm, I75mm & I9Omm thick precast panel







Installation and performance details (per anchor)

Daniel Thisleres	Reinforcing Bar D	etails Concrete Stre	Drossbach Details		
Panel Thickness b _w (mm)	Rebar Diameter d _b (mm)	Reinforcing Bar development length I _d (mm)*	Gr500E Reinforcing Bar Yield Strength f _{sy} (kN)	Drossbach Diameter Ø _{dr} (mm)	Drossbach Length L _{dr} (mm)
150	12	474	56.5	50	650
150	16	632	100.5	50	830
175	16	632	100.5	60	860
175	20	790	157.0	60	1050
190	16	632	100.5	60	860

Slab/Foundation

Note:

- Reinforcement detailing as per SESOC Interim Design Guidance (Version No. 9 26 March 2013)²
- 2. "It is recommended that the minimum grout strength be 10 MPa greater than that of the surrounding concrete" 3

^{*}I, as per Section 8.6.3.2 of NZS3101:Part 1: 2006 A3

² Reference: SESOC Interim Design Guidance (Version No. 9 -26 March 2013). ³ Reference: Guidelines for the Use of Structural Precast Concrete in Buildings (Second Edition December 1999)

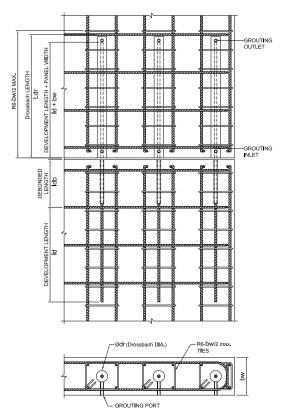


Application 5: Grouted Metal Duct Connection

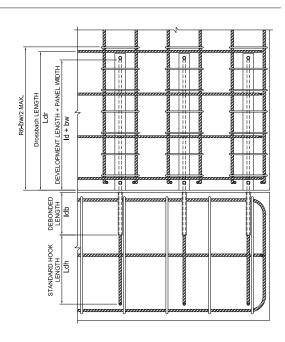
Drossbach Detail for 200mm. 225mm & 250mm thick precast panel

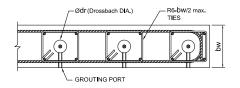
Drossbach (()

Panel to Panel



Slab/Foundation





Installation and performance details (per anchor)

Panel Thickness b _w (mm)	Reinforcing Bar D	etails Concrete Stre	Drossbach Details		
	Rebar Diameter d _b (mm)	Reinforcing Bar development length I _d (mm)*	Gr500E Reinforcing Bar Yield Strength f _{sy} (kN)	Drossbach Diameter Ø _{dr} (mm)	Drossbach Length L _{dr} (mm)
200	16	632	100.5	60	860
200	20	790	157.0	70	1050
225	16	632	100.5	60	860
225	20	790	157.0	70	1100
250	25	988	245.5	70	1300
250	32	1265	308.0	70	1600

Not∈:

- Reinforcement detailing as per SESOC Interim Design Guidance (Version No. 9 26 March 2013) ²
- "It is recommended that the minimum grout strength be 10 MPa greater than that of the surrounding concrete" 3

^{*}I_d as per Section 8.6.3.2 of NZS3101:Part 1: 2006 A3

² Reference: SESOC Interim Design Guidance (Version No. 9 -26 March 2013). ³ Reference: Guidelines for the Use of Structural Precast Concrete in Buildings (Second Edition December 1999)

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