



December 2023

Metric Threaded Inserts

Design Guide

Threaded Inserts are designed for casting into concrete and providing fixing points for the attachment of structural members and ancillary fittings to be bolted directly to the concrete structure.

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Reid™ has been supplying quality engineered products to the New Zealand precast and construction industries for the last 25 years and we have built a position of market leadership in the supply of cast-in components to the concrete construction sector.

Reid™ introduced the revolutionary SwiftLift system, involving robust cast-in anchors and safe, quick release clutches. This unique system greatly increased safety and efficiency in the transportation and placement of large concrete components such as wall panels, stairs, beams and pipes. Reid innovation continued with the development of the ReidBar™ system - ReidBar™ Couplers, Grout Sleeves and Inserts that are used with ReidBar™ - continuously threaded Grade 500E reinforcing steel produced in Australia and New Zealand.

These products are now widely used to enable fast, easy and efficient reinforcement connections on major construction projects throughout all markets. Reid's ongoing commitment to innovation and investment in better products, systems and services was further strengthened when the business became a part of the global ITW Group in 2004. This gives Reid™ people access to significant technology and business resources worldwide, the benefits of which flow to our customers. In partnership with another ITW group member - Ramset, we can deliver lifting, connection, anchorage and fixing solutions for anything built from concrete.

At ReidTM, we aim to be much more than just a supplier of components to the concrete construction industry. We work in partnership with our customers in all facets of planning, preparation, design, engineering, forming, production, rigging, lifting, anchoring and bracing... all critical stages in the safe and efficient manufacture and placement of concrete elements.

Our products help handle the physical load, whilst our professional support services help lift the risk load – each Reid™ design comes backed by the strength of ITW and our absolute commitment to delivering your project faster, safer, more efficiently. This design guide is a practical demonstration of that commitment - we hope you find it useful.



- Technical representation in all cities and regional areas
- Extensive technical data and support available
- Strict quality control systems
- Products designed in New Zealand to meet New Zealand Standards
- Products tested in New Zealand building materials
- Stand behind our product range and performance data





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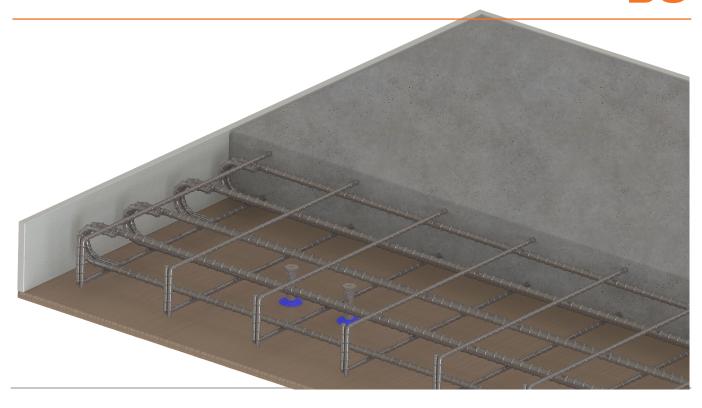
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I.I Threaded Insert Overview



Threaded Inserts are designed for casting into concrete and providing fixing points for the attachment of structural members and ancillary fittings to be bolted directly to the concrete structure. Reid supply a range of threaded inserts from MIO to M24 in zinc, hot dipped galvanised and 3I6 Stainless Steel finishes.

Pre-engineered products such as threaded inserts, require customised application / engineering to devise and deliver the best results for each project. Ultimately this produces value-added solutions for our customer's needs and inherently better engineering.

The following design information is intended to provide guidance to qualified structural engineers or other suitably skilled persons in the design of cast-in anchors and allows the designer to determine load carrying capacities based on actual application and installation conditions. With the aid of this guide, you have the option to design your cast-in solution independently, or you can easily seek the necessary support from our team of engineers or technical sales professionals. We're always here to help.





Did you know?



Extensive research, development and testing are invested in Reid™ products. This provides designers with the complete performance capabilities of the Reid range of Cast-In Metric Threaded Inserts.



The performance data contained in this Design Guide relates only to the Reid range of Cast-In Metric Threaded Inserts. Our superior steel grade and manufacturing tolerances are key factors in producing our excellent products.

Generic products may appear similar physically, but their actual performance is heavily influenced by the steel grade and manufacturing tolerances used.





Using The Design Guide

I.2 Using The Design Guide

1.21 STRENGTH LIMIT STATE DESIGN

The Simplified Design Approach to achieve strength limit state design, originally first published by our sister company Ramset, has proven to be a simple and effective method to allow for rapid selection of a suitable anchor through systematic analysis, ensuring that it will meet the required design criteria under strength limit state principles.

1.22 DEVELOPING PULLOUT OF A CONCRETE CONE

The potential to develop a full pullout cone of concrete is assumed in determining the pullout capacity of an insert. Capacity is then modified for factors such as bolt steel strength, concrete strength, spacing to other inserts, or concrete edges that prevent full development of a concrete cone. Tables are provided in the Strength Limit State section of the design guide to calculate these reductions.

1.23 REINFORCEMENT

Reinforcement in the concrete around a threaded insert can affect the concrete pullout and shear capacity. Unless it is specifically designed to work with the insert and enhance its capacity in that situation, reinforcement should always be disregarded when assessing the capacity of the insert during design. For this reason, the design tables provide loadings for inserts in unreinforced concrete without the addition of a hanger bar.

1.24 FORMULAE

Reid traditionally have applied pullout formulae for threaded inserts that have been well established and proven to reliably predict concrete pullout based on the strength of the concrete, location of the insert and the embedment depth. For the new 2023 Design Guide however, we have modified our formula to be consistent in concept with the Concrete Capacity Design (CCD) approach of embedded anchor design that NZS3101 section 17 is modelled on. The CCD approach has its origins in pullout capacity being a function of the square root of the concrete strength whereas the previous Reid pullout formula used concrete strength to the power of two thirds. For the design of the concrete shear capacity we have used NZS3101 section 17 as the basis of design.

1.25 PREVIOUS DOCUMENTATION

The change in formula will supersede any previously published Threaded Insert information from Reid or Ramset. All prior information from Reid or Ramset should be discarded as it is out of date with the publication of this design guide.

1.26 FEEDBACK

We endeavour to provide an accurate and clear document. However, if the information is not clear, inaccurate or you feel that it is misleading, please do not hesitate to contact us to discuss your concerns. We welcome industry feedback.



TIMS Product Overview

Reid™ Metric Threaded Inserts (TIMS)

Reid™ Threaded Metric Inserts (TIMS) are cast into concrete to provide fixing points for attachments and structural members. The TIM is the original Reid design with a large foot at the base of the insert (see nominal dimensions).

All TIMS with the exception of the TIM20x120 have a hole through the shaft to accept a cross bar for situations where full pullout capacity of the TIM is required in thin concrete sections or where the TIM is close to an edge.

Features and Benefits

Advantages

- Easy and simple to use
- Improved security and reliability for slip free anchorage
- No cross bar required to develop rated capacity
- Resists dynamic loading
- Plastic nail plates simplify installation and minimise fouling during the concrete pour

Versatile Applications

- Medium to heavy duty applications
- Use in near or far face applications with our range of accessories
- May be used with small rebar for fixing to mesh
- Used to provide Structural, building service & temporary connections.
- Curtain wall and panel facades
- Temporary precast panel bracing points

Finishes & Material

- High grade carbon steel
- Gold passivated zinc coating to AS1789:2003 recommended for internal applications
- Hot Dip Galvanised, recommended for external applications





Performance Details (TIMS)

Reduced characteristic ultimate bolt shear capacity ϕV_{S2} (kN), $\phi = 0.8$



			Shear, V _{s2} (kN)			
Bolt Size	Suitable Threaded Insert Product Code	Area of Root of thread (mm²)	Steel Grade of Bolt			
			4.6	8.8		
M10	TIM10X40G	52.3	9.8	20.0		
M12	TIM12X50G	76.2	14.4	29.3		
M16	TIM16X75G	144	27.4	56.1		
M20	TIM20X75G / TIM20X120G	225	43.0	88.3		
M24	TIM24X100G	324	62.0	127.2		





Performance Details (TIMS)

Site Installation and Performance Details Reid™ TIMS in Uncracked and Unreinforced Concrete

		lr	nstallation Deta	ils	Optimum D	imensions*		F	Reduced cl	haracterisit	tc capacity		
Bolt	Product	Tightening Incert or half Substrate		Shear of edge based on dimensions e		$\phi V_{uc}(kN)^{**}$ towards based on optimum Tension, ϕN_{uc} [kN nsions e_c and e_m		κN]**					
Size	Code	hole Dia, [mm]	Torque, T [Nm]	Length (L)	distance to another anchor e	Thickness b _m [mm]	8.8 Bolt (No edge Distance)		ete Compr Strength, f		Concrete Compressive Strength, f'c		
					[mm]		φV _{s2} [kN]	20MPa	30MPa	40MPa	20MPa	30MPa	40MPa
	TIM10X40G							5.3	6.5	7.5	7.9	9.6	11.1
M10	***with nail plate	9	17	40	60	50	20.0	5.6	6.8	7.9	10.6	13.0	15.0
	TIM12X50G							8.0	9.8	11.3	11.4	13.9	16.1
M12	***with nail plate	9.5	30	50	75	65	29.3	8.2	10.1	11.7	14.4	17.6	20.3
	TIM16X75G							17.9	21.9	25.2	21.8	26.6	30.8
M16	***with nail plate	11.6	75	75	115	95	56.1	18.2	22.3	25.8	25.5	31.2	36.1
	TIM20X75G							19.1	23.4	27.0	21.3	26.0	30.1
M20	***with nail plate	15.5	144	75	115	95	88.3	19.1	23.4	27.0	25.0	30.6	35.4
	TIM20X120G							37.3	45.7	52.8	44.8	54.8	63.3
M20	***with nail plate	N/A	144	120	180	150	88.3	37.3	45.7	52.8	49.6	60.8	70.2
	TIM24X100G							28.4	34.8	40.2	33.6	41.2	47.6
M24	***with nail plate	20.5	250	100	150	125	127.2	28.4	34.8	40.2	38.0	46.5	53.7

^{*} Where these minimum dimensions are not achievable, please use the simplified strength limit state design process to verify capacity



^{**}For conversion to Working Load Limit (FOS = 3), MULTIPLY $\phi N_{_{\rm HC}} \times 0.51$ for Tension

^{**}For conversion to Working Load Limit (FOS = 3), MULTIPLY $\phi V_{uc} \times 0.51$ for Shear towards edge

^{**}For conversion to Working Load Limit (FOS = 2.5), MULTIPLY $\phi V_{sp} \times 0.50$ for Shear of Gr 8.8 Bolt

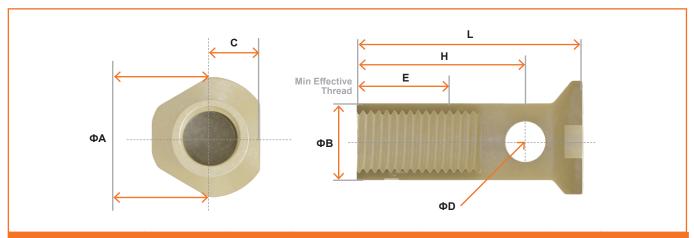
^{***} Nail Plate set insert 8mm deeper into concrete



Nominal Dimensions (TIMS)



Product Codes and Nominal Dimensions Reid Threaded Insert (TIM)



Reid™ Metric Threaded Inserts - Nominal Dimensions (mm)

Metric Threaded Inserts	Insert Length, L [mm]	ФА	ФВ	С	ΦD	E [min]	Н	Thread / Pitch
TIM10X40G	40	25	16	10.6	9	14	25	M10x1.5P
*with nail plate	48	25	10	10.6	9	14	25	WITOXT.SP
TIM12X50G	50	28	17	11	9	20	37	M12x1.75P
*with nail plate	58	20	17	- 11	9	20	31	W112X1.75P
TIM16X75 / TIM16X75G	75	39	22	16.6	11	35	55	M16x2P
*with nail plate	83	39	22	10.0				
TIM20X75 / TIM20X75G	75	53	30	23.5	15.5	32	55	M20x2.5P
*with nail plate	83	50	00	20.0		02	33	
TIM20X120G	120	50	30	N/A	N/A	35	N/A	M20x2.5P
*with nail plate	128	50	00	IV/74	IN//A	00	11/73	WEUNE.OI
TIM24X100G	100	60	38	26	20	45	76	M24x3P
*with nail plate	108	00	30	26	20			IVIZHADI

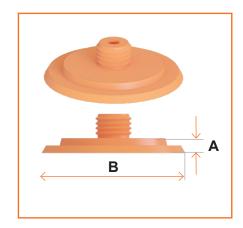
^{*} Nail Plate set insert 8mm deeper into concrete



Metric Threaded Insert Nailing Plates

Product Codes for Metric Nail Plates to suit Reid Threaded Inserts

Product Code	Description	Depth of Nail Plate (mm) (A)	Diameter of Nail Plate (mm) (B)
NP10	Metric Nail Plate M10	8mm	63mm
NP12	Metric Nail Plate M12	8mm	63mm
NP16	Metric Nail Plate M16	8mm	63mm
NP20	Metric Nail Plate M20	8mm	63mm
NP24	Metric Nail Plate M24	8mm	63mm



Metric Plastic Glue On Nail Plates

Glue on nailing plates are used to attach ferrules to steel formwork or casting beds, where screwing or drilling is not desired. Designed to be used with Reid Double Sided Spots, they have a recess on the bottom side that allows for easy removal after the panel has been lifted.

Product Code	To Suit	Pack Qty
GP12	M12	100
NP16GLUE	M16	100
GP20	M20	100
GP24	M24	100

Double Sided Spots to S	Double Sided Spots to Suit:						
Product Code	Diam Pack	Pack Qty					
NPSPOT	58mm	100					







Material Properties (TIMS)

Engineering Properties Reid™ Threaded Insert (TIMS)

			Bearing area of head	Carbon Steel *			
Size	Product Code	Min.stress area of anchor, A _s (mm²)	of insert, A _{brg} , (mm²)	Yeild Strength f _y (MPa)	UTS, f _u (MPa)		
M10	TIM10X40G	65.0	237.4	240	400		
M12	TIM12X50G	81.4	247.5	240	400		
M16	TIM16X75G	127.8	696.4	240	400		
M20	TIM20X75G	263.4	1350.6	240	400		
M20	TIM20X120G	263.4	1350.6	370**	600**		
M24	TIM24X100G	410.8	1450.5	240	400		

^{*} Mechanical Properties of Carbon Steel for all sizes except TIM20x120.

Installation Instructions (TIMS)

- Either insert a positioning bolt through the mould wall or boxing and thread the insert onto the bolt until flush with the wall, or thread the insert onto a nail plate and fix this to the mould.
- If required, pass a rebar of the correct diameter and length through the cross-hole in the insert and tie to the reinforcement to prevent it moving during pouring and vibration of the concrete.
- When the concrete has cured, remove the bolt and mould. If a nailing plate has been used leave it screwed into the insert until immediately before use to help protect the internal thread.

Limitations

- Reid[™] Threaded Inserts MUST NOT be used for lifting. Contact your local Reid[™] Engineer or sales person for advice on lifting designs & products.
- Structural capacity is dependent upon the load capacity required, or the diameter of the fixing bolt selected.
- Remember the practical aspect: small diameter inserts are much more prone to fouling and thread damage than larger inserts. For most applications it is preferable to use inserts of M12 diameter or greater.



1. Nailing plate or bolted to formwork



2. Fixing to steel casting bed with glue-on nailing plate



3. "Puddled" into wet concrete



4. Templated onto face of panel

^{**} Mechanical Properties of TIMx120 is based on SG IRON QT 600-3.



TCM Product Overview

Reid™ 3I6 Stainless Steel Metric Threaded Inserts (TCM)

The Reid™ Stainless Steel Metric Threaded Inserts (TCM) are manufactured from premium grade 316 Stainless Steel and suit medium to heavy duty structural applications. Noticably different from the TIMS, this threaded insert has an Elephant Foot™.

All TCMs have a hole through the shaft to accept a cross bar for situations where full pullout capacity of the TCMs is required. ie thin concrete sections or where the TCM is close to an edge.

Features and Benefits

Advantages

- Easy and simple to use
- Improved security and reliability for slip free anchorage
- No cross bar required to develop rated capacity
- Resists dynamic loading
- Plastic nail plates simplify installation and minimise fouling during the concrete pour

Versatile Applications

- Suitable for use in corrosive areas where durability is required such as salt spray zones
- Use in near or far face applications with our range of accessories
- May be used with small rebar for fixing to mesh
- Used to provide Structural, building service & temporary connections.
- Curtain wall and panel facades
- Temporary precast panel bracing points

Finishes

AISI 316 Stainless Steel suitable for marine or corrosive environments



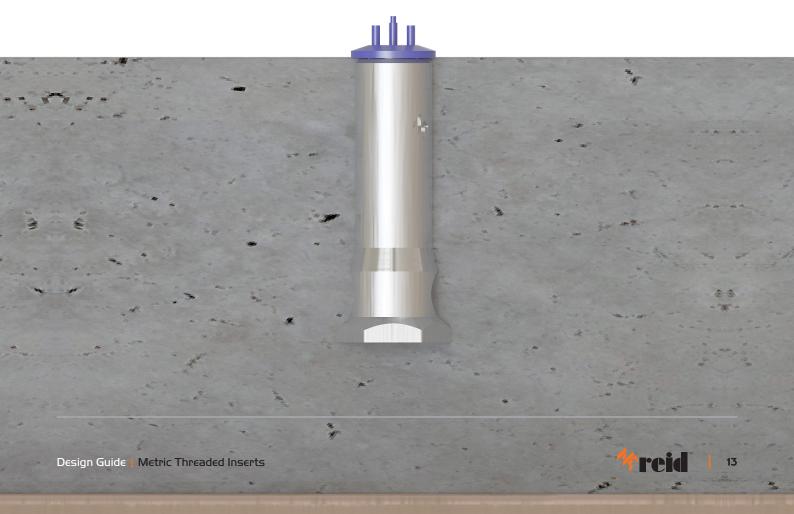


Performance Details (TCM)

Reduced characteristic ultimate bolt shear capacity ϕV_{S2} (kN), ϕ = 0.8



			Shear, V _{s2} (kN)
Bolt Size	Suitable Threaded Insert Product Code	Area of Root of thread (mm²)	Steel Grade of Bolt
	msert rioduct code		AISI 316 Stainless Steel
M10	FE10045SS	52.3	16.8
M12	FE12055SS	76.2	24.7
M16	FE16070SS	144	47.4
M20	FF20095SS	225	74.5





Site Installation and Performance Details 316 Stainless Steel FE in Uncracked and Unreinforced Concrete

		li	nstallation Detai	ls	Optimum D	imensions*		I	Reduced c	haracterisit	c capacity			
Bolt Size	Product Code	Cross	Tightening	Insert	Edge Distance or half	Substrate	Shear For AISI Substrate 316 SS		Shear, $\phi V_{uc}(kN)^{++}$ towards edge based on optimum dimensions e_c and b_m			Tension, φN _{uc} [kN]**		
OIZE	Code	hole Dia, [mm]	Torque, T [Nm]	Length (L)	distance to another anchor e _c [mm]	Thickness b _m [mm]		Concrete Compressive Strength, f'c			Concrete Compressive Strength, fc			
							· s2 · ·	20MPa	30MPa	40MPa	20MPa	30MPa	40MPa	
	FE10045SS							5.5	6.7	7.7	9.5	11.6	13.4	
M10	***with nail plate	8.4	17	45	60	50	16.8	5.7	7.0	8.0	12.5	15.3	17.7	
	FE12055SS							8.1	10.0	11.5	13.3	16.2	18.7	
M12	***with nail plate	8.4	30	55	75	65	24.7	8.4	10.3	11.9	16.5	20.2	23.3	
	FE16070SS							13.9	17.0	19.6	19.5	23.9	27.6	
M16	***with nail plate	11	75	70	100	85	47.4	14.2	17.4	20.1	23.1	28.3	32.7	
	FE20095SS							23.9	29.2	33.7	31.0	38.0	43.8	
M20	***with nail plate	14.1	144	95	135	115	74.5	24.2	29.7	34.3	35.3	43.2	49.9	

^{*} Where these minimum dimensions are not achievable, please use the simplified strength limit state design process to verify capacity



^{**}For conversion to Working Load Limit (FOS = 3), MULTIPLY $\phi N_{uc} \times 0.51$ for Tension

^{**}For conversion to Working Load Limit (FOS = 3), MULTIPLY $\phi V_{uc} \times 0.51$ for Shear towards edge

^{**}For conversion to Working Load Limit (FOS = 2.5), MULTIPLY $\phi V_{s2} \times 0.50$ for Shear of AISI 316 SS Bolt

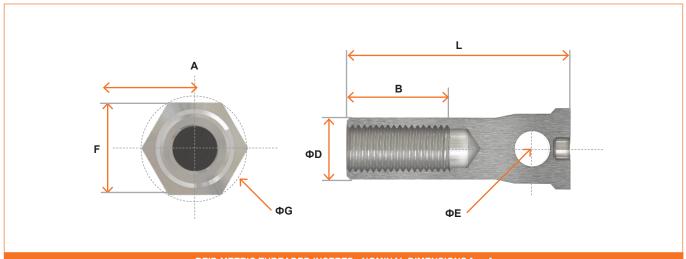
^{***} Nail Plate set insert 8mm deeper into concrete



Nominal Dimensions (TCM)



Product Codes and Nominal Dimensions Reid Threaded Insert (TCM)



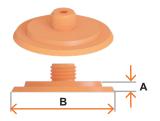
REID METRIC THREADED INSERTS - NOMINAL DIN	MENSIONS [mm]

REID Metric Threaded Inserts	Insert Length, L [mm]	А	В	ΦD	ΦЕ	F	ФС	Thread / Pitch
FE10045SS	45	M10	20	16	8.4	19.0-19.3	20.9	M10x1.5P
*with nail plate	53	WHO	20	10	0.4	19.0-19.5	20.9	WITOX 1.5F
FE12055SS	55	M12	25	17	8.4	22.0-22.3	24.4	M12x1.75P
*with nail plate	63	IVITZ	25	17	0.4	22.0-22.3	24.4	WITZXT.75F
FE16070SS	70	M16	32	22	14.5	31.8	35.0	M16x2P
*with nail plate	78	IVITO	32	22	14.5	31.0	35.0	W TOXZP
FE20095SS	95	M20	38	26	14.5	34.5	37.0	M20v2 ED
*with nail plate	103	IVIZU	30	20	14.5	54.5	37.0	M20x2.5P

^{*} Nail Plate set insert 8mm deeper into concrete

Product Codes for Metric Nail Plates to suit Reid Threaded Inserts

Product Codes	Description	Depth of Nail Plate (mm) (A)	Diameter of Nail Plate (mm) (B)
NP10	Metric Nail Plate M10	8	63
NP12	Metric Nail Plate M12	8	63
NP16	Metric Nail Plate M16	8	63
NP20	Metric Nail Plate M20	8	63
NP24	Metric Nail Plate M24	8	63



Note: 'Stick-on' nail plates available to suit most sizes. Refer to 'Metric Plastic Glue-On Nail Plate' table on Page 10 for further details.



Material Properties (тсм)

Engineering Properties Reid Threaded Insert (TCM)

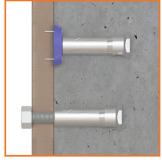
T 11 01	2	Min.stress area of anchor,	AISI 316 Stainless Steel			
Bolt Size	Product Code	A _s (mm²)	Yield strength, f _y (MPa)	UTS, f _u (MPa)		
M10	FE10045SS	71.3	450	700		
M12	FE12055SS	88.3	450	700		
M16	FE16070SS	158.0	450	600		
M20	FE20095SS	242.0	450	600		

Installation Instructions (TCM)

- Either insert a positioning bolt through the mould wall or boxing and thread the insert onto the bolt until flush with the wall, or thread the insert onto a nail plate and fix this to the mould.
- If required, pass a rebar of the correct diameter and length through the cross-hole in the insert and tie to the reinforcement to prevent it moving during pouring and vibration of the concrete.
- When the concrete has cured, remove the bolt and mould. If a nailing plate has been used leave it screwed into the insert until immediately before use to help protect the internal thread.

Limitations

- Reid Threaded Inserts MUST NOT be used for lifting. Contact your local Reid Engineer or sales person for advice on lifting designs & products.
- Structural capacity is dependent upon the load capacity required, or the diameter of the fixing bolt selected.
- Remember the practical aspect: small diameter inserts are much more prone to fouling and thread damage than larger inserts. For most applications it is preferable to use inserts of M12 diameter or greater.



1. Nailing plate or bolted to formwork



2. Fixing to steel casting Ded with glue-on nailing plate



3. "Puddled" into wet concrete



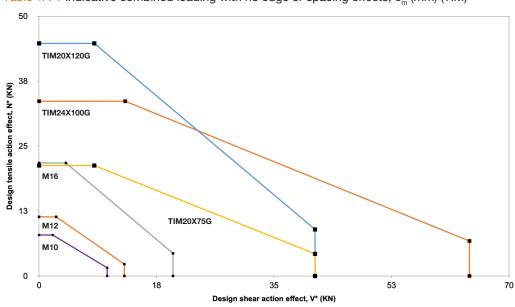
4. Templated onto face of panel



Strength Limit State Design

Step I. Select Anchor to be Evaluated

Table 1A-1 Indicative combined loading with no edge or spacing effects, e_ (mm) (TIM)

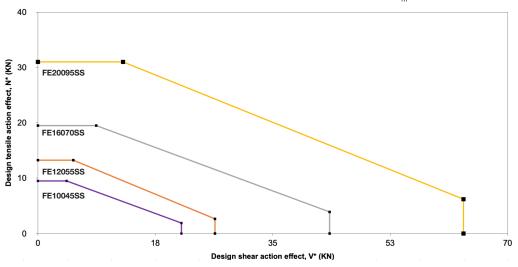




Notes:

- Tension limited by the lesser of steel capacity and concrete cone capacity. No edge ors spacing effects.
- Shear limited by the TIM capcity
- 3. f_c = 20 MPa
- Without Nail Plate

Table 1A-2 Indicative combined loading with no edge or spacing effects, e_m (mm) (TCM 316SS)



FE10045SS
FE12055SS
FE16070SS
FE20095SS

Table 1A Absolute minimum edge distance and half of anchor spacing values, e_ (mm)

Product Code	TIM10X40G FE10045SS	TIM12X50G FE12055SS	TIM16X75G FE16070SS	TIM20X75G FE20095SS	TIM20X120G	TIM24X100G
e _m , mm	30	36	48	60	85	72



Anchor Size, Absolute Minima Compliance Achieved



Step 2. Verify Concrete Tensile Capacity $\varphi N_{_{\! U\!R\!C}}$ (per anchor)

Table 2A Reduced characteristic ultimate concrete tensile capacity, ϕN_{uc} (kN), ϕ = 0.65, f'c = 20 MPa

Product Code	TIM10X40G	TIM12X50G	TIM16X75G	TIM20X75G	TIM20X120G	TIM24X100G
φN _{uc} [kN]	7.9	11.4	21.8	21.3	44.8	33.6
with nail plate	10.6	14.4	25.5	25.0	49.6	38.0
Product Code	FE10045SS	FE12055SS	FE16070SS	FE20095SS		
Product Code φN _{uc} [kN]	FE10045SS 9.5	FE12055SS 13.3	FE16070SS 19.5	FE20095SS 31.0		

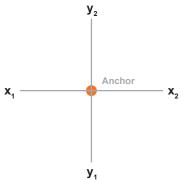
Table 2B-1 Cracked concrete effect, tension, ψ_{ner}

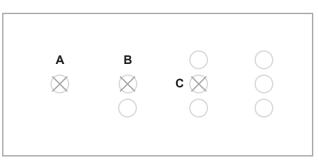
Product Code	TIM10X40G	TIM12X50G	TIM16X75G	TIM20X75G	TIM20X120G	TIM24X100G
$\Psi_{\sf ncr}$			0	.8		
Product Code	FE10045SS	FE12055SS	FE16070SS	FE20095SS	Table 2B-1 Note: For concrete $\psi_{ncr} = 1.0$	Non-cracked
Ψ_{ncr}		0	.8		1103	

Table 2B-2 Concrete compresive strength effect, tension, ψ_{nc}

f'c [MPa]	15	20	25	30	40	50
$\Psi_{\sf nc}$	0.87	1	1.12	1.22	1.41	1.58

Diagram 2 Edge and spacing effect, tension, $x = \phi x_1^* \phi x_2^* \phi y_1^* \phi y_2^*$





Examples

Apply edge and spacing reduction factors as appropriate to the anchor location

Anchor —	1	A	В		С	
Factor	Туре	Table	Туре	Table	Туре	Table
φx ₁	Edge	2c	Spacing	2d	Spacing	2d
$\phi x_{_2}$	Spacing	2d	Spacing	2d	Spacing	2d
$\phi y_{_1}$	Edge	2c	Spacing	2d	Spacing	2d
ϕy_2	Edge	2c	Edge	2c	Spacing	2d

If edge distance to anchor greater than 1.5 times effective depth then reduction factor = 1.0. If anchor spacing greater than 3 times efective depth then reduction factor = 1.0.





Table 2C Edge distance reduction factor $\varphi_{\mbox{\tiny ex}}$ and $\varphi_{\mbox{\tiny ey}}$

Bud at Oak				And	chor Edge D	istance, e (n	nm)			
Product Code	30	40	50	60	70	85	100	125	150	>200
TIM10X40G *with nail plate	0.74 0.69	0.84 0.77	0.95 0.85	1.00 0.94	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
TIM12X50G *with nail plate		0.75 0.71	0.83 0.77	0.92 0.84	1.00 0.91	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
TIM16X75G *with nail plate			0.70 0.67	0.74 0.72	0.79 0.76	0.87 0.83	0.96 0.90	1.00 1.00	1.00 1.00	1.00 1.00
TIM20X75G *with nail plate			0.70 0.68	0.75 0.72	0.80 0.76	0.88 0.83	0.97 0.91	1.00 1.00	1.00 1.00	1.00 1.00
TIM20X120G *with nail plate						0.71 0.69	0.75 0.73	0.83 0.81	0.92 0.88	1.00 1.00
TIM24X100G *with nail plate					0.71 0.69	0.76 0.74	0.82 0.79	0.92 0.88	1.00 0.98	1.00 1.00
316 Stainless Steel Ins	erts (TCM)									
FE100145SS *with nail plate	0.71 0.67	0.79 0.73	0.88 0.81	1.00 0.97	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FE12055SS *with nail plate		0.72 0.69	0.79 0.75	0.87 0.81	0.95 0.87	1.00 0.97	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FE16070SS *with nail plate			0.72 0.69	0.77 0.73	0.82 0.78	0.91 0.86	1.00 0.94	1.00 1.00	1.00 1.00	1.00 1.00
FE20095SS *with nail plate			0.65 0.64	0.68 0.67	0.72 0.70	0.78 0.75	0.84 0.81	0.95 0.91	1.00 1.00	1.00 1.00



Table 2D Anchor spacing distance reduction factor φ_{sx} and φ_{sy}

D. J. J. O. J.			1			And	chor spac	cing, a (m	nm)					
Product Code	30	40	50	60	70	85	100	125	150	200	250	300	350	>400
TIM10X40G *with nail plate	0.64 0.61	0.69 0.65	0.73 0.69	0.78 0.73	0.82 0.77	0.89 0.82	0.96 0.88	1.00 0.97	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
TIM12X50G *with nail plate		0.64 0.62	0.68 0.65	0.72 0.69	0.75 0.72	0.81 0.76	0.86 0.81	0.95 0.89	1.00 0.96	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
TIM16X75G *with nail plate			0.62 0.61	0.64 0.63	0.67 0.65	0.70 0.68	0.74 0.71	0.80 0.77	0.86 0.82	0.98 0.93	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
TIM20X75G *with nail plate			0.62 0.61	0.64 0.63	0.67 0.65	0.70 0.68	0.74 0.71	0.80 0.77	0.86 0.82	0.98 0.93	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
TIM20X120G *with nail plate						0.62 0.62	0.64 0.64	0.68 0.67	0.72 0.70	0.76 0.74	0.85 0.82	0.94 0.90	0.10 0.99	1.00 1.00
TIM24X100G *with nail plate					0.62 0.61	0.65 0.64	0.68 0.66	0.72 0.70	0.76 0.74	0.85 0.82	0.94 0.90	0.10 0.99	1.00 1.00	1.00
316 Stainless Ste	el Threa	aded Ins	erts (TC	M)										
FE100145SS *with nail plate	0.62 0.60	0.66 0.64	0.70 0.67	0.74 0.70	0.78 0.74	0.85 0.79	0.91 0.84	1.00 0.93	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FE12055SS *with nail plate		0.63 0.61	0.66 0.64	0.70 0.67	0.73 0.70	0.78 0.74	9.83 0.78	0.91 0.85	0.99 0.92	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FE16070SS *with nail plate			0.63 0.61	0.65 0.64	0.68 0.66	0.71 0.69	0.75 0.73	0.82 0.78	0.88 0.84	1.00 0.95	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FE20095SS *with nail plate			0.59 0.59	0.61 0.60	0.63 0.62	0.66 0.64	0.68 0.67	0.73 0.71	0.77 0.76	0.87 0.84	0.96 0.93	1.00 1.00	1.00 1.00	1.00 1.00

Verify Concrete Tension Capacity

Computation 2E Complete the table and determine, X as the product of the factors

Factor	Туре	Table	Value
φx ₁			
фх ₂			
фу ₁			
фу ₂			
Fa			



Determine the reduced ultimate concrete tensile capacity of the anchor situation ϕN_{urc}

$$\Phi N_{urc} = \Phi N_{uc} \hat{\Psi}_{ncr} \hat{\Psi}_{nc} \hat{X}$$



Step 3. Verify Steel Tensile Capacity, $\varphi N_{_{US}}\left(kN\right)$ (per anchor)

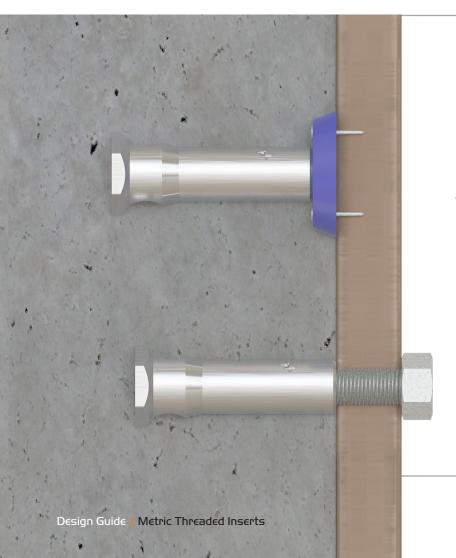
Table 3A Reduced characteristic ultimate anchor steel capacity, ϕN_{us} (kN), $\phi = 0.75$

Product Code	TIM10X40G	TIM12X50G	TIM16X75G	TIM20X75G	TIM20X120G	TIM24X100G
φN _{us} [kN]	19.5	24.4	38.3	79.0	79.0	123.2
Product Code	FE10045SS	FE12055SS	FE16070SS	FE20095SS		
φN _{us} [kN]	37.3	46.3	71.1	108.9		

Table 3B Reduced characteristic ultimate bolt tensile capacity, ϕN_{TF} (kN), $\phi_n = 0.8$

G4.6 G8.8 G4.6 G8.8 G4.6 G8.8 G4.6 G8.8 G4.6	
	G8.8
φNTF (kN) 18.6 38.5 27 56 50.2 104.2 78.4 162.7 112.8	234.4

Bolt Size AISI 316 SS	M10	M12	M16	M20
φN _{τF} [kN]	32.5	47.2	87.9	137.2



Verify Steel Tension Capacity

Step 3 Establish the reduced characteristic bolt tensile capacity, from the literature of the specified bolt manufacturer. Table 3B details the nominal expected capacities of bolts manufactured to ISO standards.



Design reduced ultimate tensile capacity, $\phi N_{ur}(kN)$

 $\phi N_{ur} = Minimum of \phi N_{urc} \phi N_{US} \phi N_{TF}$

Check $N^* / \phi N_{ur} \leq 1$,

If not satisfied return to Step I



Step 4. Verify Concrete Shear Capacity (per anchor) Perpendicular to Edge, ϕV_{CB} (kN), $\phi=0.65$

Anchor Shear Capacity (reference NZS3101 Section 17)

Table 4A Reduced basic concrete breakout strength in Shear, per anchor ϕV_B (kN), for single anchor $\phi = 0.65 f_c = 20 MPa$

	c1 - dist	tance from	the centr	e of resista	ance of an	anchor to	the edge	of the con	crete in the	e direction	which the	e load is
Product Code	30	35	40	50	60	85	100	200	300	400	500	≥ 600
TIM10X40G *with nail plate	1.9 2.0	2.4 2.5	2.9 3.0	4.1 4.2	5.3 5.6	9.0 9.4	11.5 12.0	32.5 33.8	59.7 62.1	91.9 95.7	128.4 133.7	168.8 175.7
TIM12X50G *with nail plate		2.5 2.6	3.1 3.2	4.3 4.5	5.7 5.9	9.6 9.9	12.3 12.7	34.7 35.9	63.8 65.9	98.3 101.5	137.4 141.8	180.6 186.4
TIM16X75G *with nail plate				5.1 5.2	6.7 6.9	11.3 11.6	14.5 14.8	40.9 41.8	75.2 76.8	115.8 118.3	161.9 165.4	212.8 217.4
TIM20X75G *with nail plate					7.2 5.4	12.1 9.1	15.5 11.6	43.7 32.7	80.4 60.1	123.7 92.5	172.9 129.3	227.3 170.0
TIM20X120G *with nail plate						12.1 12.1	15.5 15.5	43.7 43.7	80.4 80.4	123.7 123.7	172.9 172.9	227.3 227.3
TIM24X100G *with nail plate						12.1 12.1	15.5 15.5	43.7 43.7	80.4 80.4	123.7 123.7	172.9 172.9	227.3 227.3
316 Stainless Steel	Threaded	d Inserts	(TCM)									
FE100145SS *with nail plate	1.9 2.0	2.4 2.5	3.0 3.1	4.2 4.3	5.5 5.7	9.2 9.6	11.8 12.2	33.3 34.6	61.3 63.5	94.3 97.7	131.8 136.6	173.3 179.6
FE12055SS *with nail plate		2.6 2.7	3.2 3.3	4.4 4.6	5.8 6.0	9.8 10.1	12.5 12.9	35.5 36.5	65.2 67.1	100.3 100.3	140.2 144.4	184.3 189.8
FE16070SS *with nail plate			3.5 3.6	4.9 5.0	6.4 6.6	10.9 11.1	13.9 14.2	39.2 40.1	72.0 73.7	110.9 113.5	155.0 158.6	203.8 208.5
FE20095SS *with nail plate					7.1 7.2	11.9 12.1	15.2 15.4	43.0 43.7	79.1 80.2	121.7 123.5	170.1 172.6	223.6 226.9

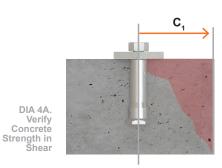


Table 4B-1 Cracked concrete effect, shear, ψ_{vcr}

			· · · VCI								
Product Code	TIM- 10X40G	TIM- 12X50G	TIM- 16X75G	TIM- 20X75G	TIM- 20X120G	TIM- 24X100G					
ψ_{vcr}		0.7									
Product Co	ode FE10	FE10045SS FE12055SS FE16070SS FE20									
ψ_{vcr}		0.7									

Note: For Non-cracked concrete ψ_{vcr} =1



Table 4B-2 Concrete compresive strength effect, shear, $\psi_{\mbox{\tiny VC}}$

f'c [MPa]	15	20	25	≥ 30
Ψ_{ncr}	0.87	1	1.12	1.22

Table 4C Modification factor for anchor groups, $\Psi_{_{5}}$ if $e_{_{v}}$ <half of centre-to-centre spacing, s)

	C ₁ - dist	ance fron	n the cen	tre of resi	stance of	an ancho	or to the e	edge of th	e concret	te in the d	lirection v	which the	load is
e _v -distance between	#	30	35	40	50	60	85	100	200	300	400	500	600
	0	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	40	0.53	0.57	0.60	0.65	0.69	0.76	0.79	0.88	0.92	0.94	0.95	0.96
the point of shear application and	80	0.36	0.40	0.43	0.48	0.53	0.61	0.65	0.79	0.85	0.88	0.90	0.92
the centroid of the group of anchors	120	0.27	0.30	0.33	0.38	0.43	0.52	0.56	0.71	0.79	0.83	0.86	0.88
resisting the shear in direction of the	160	0.22	0.25	0.27	0.32	0.36	0.44	0.48	0.65	0.74	0.79	0.82	0.85
applied shear	200	0.18	0.21	0.23	0.27	0.31	0.39	0.43	0.60	0.69	0.75	0.79	0.82
	240	0.16	0.18	0.20	0.24	0.27	0.35	0.38	0.56	0.65	0.71	0.76	0.79
	280	0.14	0.16	0.18	0.21	0.24	0.31	0.35	0.52	0.62	0.68	0.73	0.76
	320	0.12	0.14	0.16	0.19	0.22	0.28	0.32	0.48	0.58	0.65	0.70	0.74

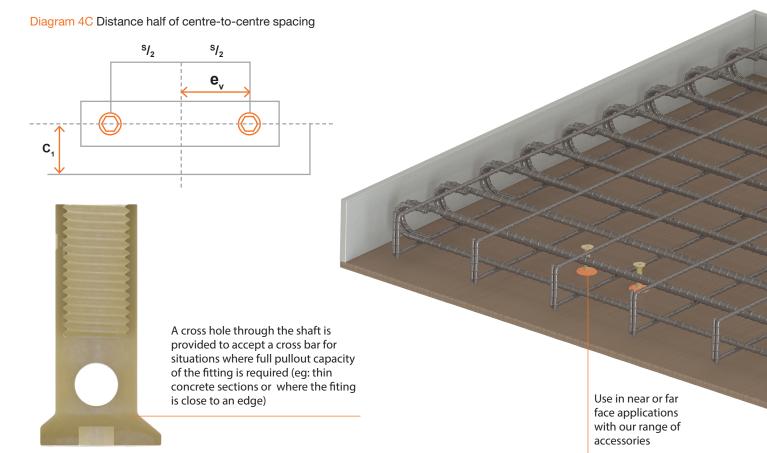




Table 4D Modification factor for anchor groups, edge distance, $\Psi_{_6}$

		c ₁ - dista	ance from	the centre	e of resista	nce of an	anchor to	the edge o	of the cond	rete in the	direction	which the	e load is
		30	35	40	50	60	85	100	200	300	400	500	≥ 600
	30	0.90	0.87	0.85	0.82	0.80	0.77	0.76	0.73	0.72	0.72	0.71	0.71
	35	0.93	0.90	0.88	0.84	0.82	0.78	0.77	0.74	0.72	0.72	0.71	0.71
c ₂ - mini-	40	0.97	0.93	0.90	0.86	9.83	0.79	0.78	0.74	0.73	0.72	0.72	0.71
mum edge	50	1.00	0.99	0.95	0.90	0.87	0.82	0.90	0.75	0.73	0.73	0.72	0.72
dis- tance or	60		1.00	1.00	0.94	0.90	0.84	0.82	0.76	0.74	0.73	0.72	0.72
half of anchor	70			1.00	0.98	0.93	0.86	0.84	0.77	0.75	0.74	0.73	0.72
spacing perpen-	100				1.00	1.00	0.94	0.90	0.80	0.77	0.75	0.74	0.73
dicular to c,	200						1.00	1.00	0.90	0.83	0.80	0.78	0.77
mm ,	300								1.00	0.90	0.85	0.82	0.80
	450									1.00	0.94	0.88	0.85
	600										1.00	0.94	0.90
	750											1.00	0.95
	1000												1.00

Diagram 4D Distance from the centre of resistance to the edge

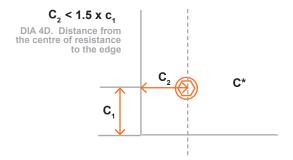


Table 4E Calculate the projected concrete failure area of anchor in shear when not limited by edge distance $A_{v0} = 4.5c_1^2$

C ₁	30	35	40	50	60	70	100	200	300	400	500	600
A _{v0} (x10 ⁴)	0.4	0.6	0.7	1.1	1.6	2.2	4.5	18.0	40.5	72.0	112.5	162.0

Table 4F Calculate 1.5c,

C ₁	30	35	40	50	60	70	100	200	300	400	500	600
1.5c ₁	45	52.5	60	75	90	105	150	300	450	600	750	900

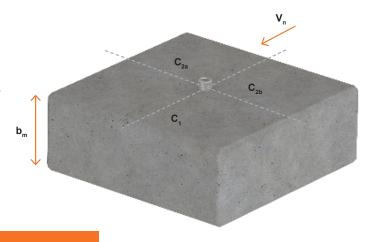


Computation 4H Calculate the project concrete failure area of anchor in shear

$$A_{v} = (c_{2a} + c_{2b}) * b_{m}, (c_{2a}, c_{2b}, b_{m} \le 1.5c_{1})$$

 c_{2a} , c_{2b} = Edge distance or half of anchor spacing perpendicular to the loading direction (but less than 1.5 c_1).

 $b_m = Substrate Thickness (but less than 1.5c_1)$



Checkpoint 4

Design Concrete Breakout Strength shear load towards edge of a single anchor, ϕV_{CB}

$$\phi V_{CB} = \phi V_{B^*} \psi_{VC}^* \psi_{VC}^* \psi_5^* \psi_{6A}^* \psi_{6B}^* A_V / A_{VO}^*$$

For when shear load direction is parrallel to edge (i.e load angle = 180°) of a singular anchor, MULTIPLY $\phi V_{CB} \times 2$

Step 5. Verify Shear Strength of Steel $\phi V_s(kN)$

Table 5A Reduced characteristic ultimate anchor steel shear capacity, φV_{s1} (kN), φ =0.65

Product Code	TIM10X40G	TIM12X50G	TIM16X75G	TIM20X75G	TIM20X120G	TIM24X100G
ϕV_{s1} [kN]	10.1	12.7	19.9	41.1	41.1	64.1
Product Code	FE10045SS	FE12055SS	FE16070SS	FE20095SS		
φV _{s1} [kN]	21.0	26.1	40.0	61.3		

Table 5B Reduced characteristic ultimate bolt steel shear capacity, ϕV_{s2} (kN), ϕ =0.80

Zinc Bolt Size	М	10	M12		M16		M20		M24	
المار المار	G4.6	G8.8								
φV _{s 2} [kN]	9.8	20.0	14.4	29.3	27.4	56.1	43.0	88.3	62.0	127.2

AISI 316 SS Bolt Size	M10	M12	M16	M20
φV _{s2} [kN]	16.8	24.7	47.4	74.5



Step 6. Verify Concrete Pry-Out Strength In Shear, ϕV_{cp} (Kn), ϕ = 0.65

Table 6A Co-efficient of pry out strength

h = length of insert (mm) plus 8mm for nail plate if used

h	<65mm	≥65mm
\mathbf{k}_{cp}	1.0	2.0



Concrete Pry-Out Strength in Shear

 $\phi V_{cp} = k_{cp} * \phi N_{urc}$

Checkpoint 6

Design Reduced Ultimate Concrete Shear Capacity Perpendicular to an Edge, $\phi V_{\mu r}$

 $\phi V_{ur} = MINIMUM OF \phi V_{CB}$, ϕV_{S1} , ϕV_{S2} , ϕV_{CD}

If not satisfied return to Step I

Step 7. Combined Loading & Specification

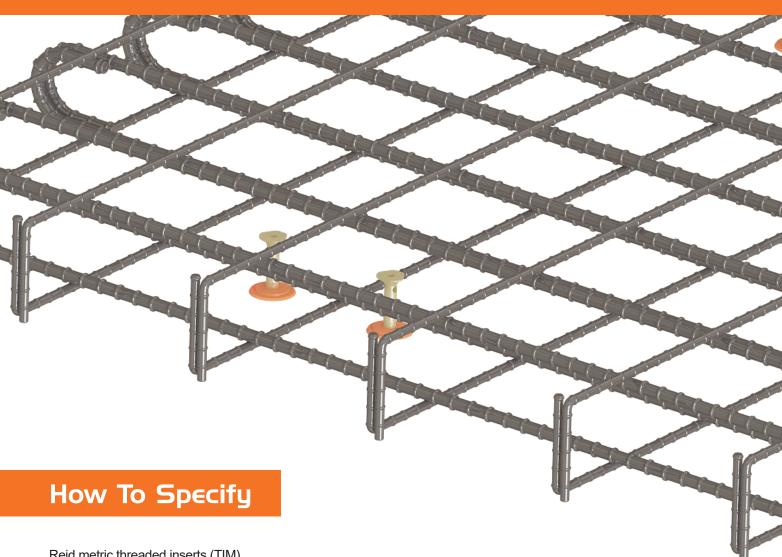


Check

 $N^*/\phi N_{ur} + V^*/\phi V_{ur} \le 1.2$,

If not satisfied return to Step I





Reid metric threaded inserts (TIM),

(TIM size x length) (part number), (finish) with a (bolt grade) bolt.

Reid 316 stainless steel elephant foot™ metric threaded inserts (fe size x length) (part number) with a (bolt grade) bolt.

Example

Reid Metric Threaded Insert 16mm x 75mm long. TIM16X75G, Galvanised Finish With a Grade 4.6 Bolt. To Be Installed In Accordance With Reid Technical Data Sheet.

| Metric Threaded Inserts Anchoring Design Worksheet



Project	Sketch						
Design							
Location							
Project ID							
Design by							
Date							
Checked							
onecica							
Inputs	STI	EP 4 VE	ERIFY CO	NCRETE	SHEAR C	APACITY	
N* & V* are the per anchor load demand. Check both external and internal anchors for suitability.	Tab	le 4a:	Concrete sh	ıear capaci	ty	φV _B	kN
Tensile design action effect N* kN	Tabl		Concrete co			Ψ _{VC}	
Shear design action effect V* kN	Tabl		Anchor grou			Ψ ₅	\dashv
Substrate thickness b _m mm			Edge distan				\dashv
Concrete compressive strength f' _c MPa			Lago alotar.			Ψ _{6A} Ψ _{6B}	\dashv
STEP 1 SELECT ANCHOR TO BE EVALUATED	Tabl	le 4e	Concrete cr	ack failure		Ψ ₇	
Table 1a: Find the interaction of N* and V* values	Tabl	le 4f	Projected fa	ilure area		A,0	mm²
Anchor Type			when not lir distance	nited by ed	ge	VU	_
Table 1a: Absolute minima, edge distance and half			Projected co	oncrete fail	ure area of		_
of anchor spacing.		dge distand Inchor spac		Original value, A	Table 4g, 1.5c₁ <u>B</u>	Calc value, min (A,B)	
e _m		C _{2a}			1—		\dashv
Check for compliance with absolute minima tick	1 🗀	C _{2b}					
Checkpoint 1:		b _m					
Anchor size selected?	1 🗆			$A_v = \{c$	C _{2a} + C _{2b})*b _m		
Comply with absolute minima?	Che	eckpoint 4:	Calculate	è			
STEP 2 VERIFY CONCRETE TENSILE CAPACITY	ф۷	$V_{CB} = \Phi V_B^* \Psi$	ν _{νC} *Ψ ₅ *Ψ _{6A} *ι	ψ _{6B} *Ψ ₇ *Α _\ /Δ	√ 0		kN 4
Table 2a: Concrete tensile capacity ϕN_{UC}	kN STI	EP 5 VI	ERIFY STI	EEL SHE	AR STRE	NGTH ∳V₅	
Table 2b: Concrete compressive strength effect ψ_{nc}	Tab	le 5a: Si	teel shear c	apacity	ф۷	s1	kN 5
The Land of the La	J Tab	le 5b: B	olt shear ca	pacity	ф۷		kN ⁶
Table 2c and Edge distance and anchor and anchor	ST		ERIFY CO		PRY-0U	T STRENGT	.H
spacing preduction factors ϕX_2 ϕY_1] Tah		ry-out co-ei			(_{cp}	7
фУ ₂	-	eckpoint 5:	,			`cp	_
Step 2a: Cracked concrete reduction factor ψ_{CR}	1		_{sp} = 0.75/0.6*	*k_, *фN,,,,,,	*Фус		kN 7
Checkpoint 2:	-	eckpoint 6:					_
Calculate: $\phi N_{URC} = \phi N_{UC} * \psi_{nc} * \phi x_1 * \phi x_2 * \phi y_1 * \phi y_2 * \psi_{CR}$	kN		4	5 6	7 ()\/	_	kN
STEP 3 VERIFY STEEL TENSILE CAPACITY		_{UR} = IMIIIIIII / φV _{ur} ≤1.0?	um of ¢ V _{cs} ,	Ψν _{S1} , Ψν _{S2} ,	ΦV _{CP}	<u>_</u>	KIN
Table 3a: Anchor steel capacity ϕN_{us}] kN	/ Ψν _{ur} < 1.0 :		/			
Table 3b: Bolt steel capacity φN _{TF}	kN ST	EP7 C	OMBINE	LOADIN	NG & SPE	CIFICATION	N
Checkpoint 3: 1 2 3 $\phi N_{UR} = minimum of \phi N_{URC}, \phi N_{US}, \phi N_{TF} =$		eckpoint 7: / φΝ _{υR} + V*	: '/φV _{UR} ≤1.2′	?		¬⊏	٦
1111 / 1111 / 100	ck C	/	+	/	=	tick	_
If not satisfied, return to step 1. Tensile Design Com	¥ If n	ot satisfied	d, return to s	step		•	
CDEOLEV							





Product Compliance

Compliance statement

Reid™ Metric Threaded Inserts comply with the New Zealand Building Code clauses identified below.

Compliance details: NZBC

NZBC Clause	Criteria	Compliance Status
B1.3.1	'Buildings, building elements and sitework shall have a low probability of rupturing, becoming unstable, losing equilibrium, or collapsing during construction or alteration and throughout their lives.'	\bigcirc
B1.3.2	'Buildings, building elements and sitework shall have a low probability of causing loss of amenity through undue deformation, vibratory response, degradation, or other physical characteristics throughout their lives, or during construction or alteration when the building is in use.'	
B1.3.3 (a), (b), (d), (e), (f), (g), (h), (j), (q)	'Account shall be taken of all physical conditions likely to affect the stability of buildings, building elements and sitework, including: (a) Self weight, (b) Imposed gravity loads arising from use \ldots (d) Earth pressure, (e) Water and other liquids, (f) Earthquake, (g) Snow, (h) Wind \ldots (j) Impact \ldots (q) Time dependent effects including creep and shrinkage.	\bigcirc
B1.3.4	'Due allowance shall be made for: (a) The consequences of failure, (b) The intended use of the building, (c) Effects of uncertainties resulting from construction activities, or the sequence in which construction activities occur, (d) Variation in the properties of materials and the characteristics of the site, and (e) Accuracy limitations inherent in the methods used to predict the stability of buildings.'	\bigcirc
B2.3.1	'Building elements must, with only normal maintenance, continue to satisfy the performance requirements of this code for the lesser of the specified intended life of the building, if stated, or: (a) The life of the building, being not less than 50 years, if (i) Those building elements Provide structural stability to the building, or (ii) Those building elements are difficult to access or replace, or (iii) Failure of those building elements to comply with the building code would go undetected during both normal use and maintenance of the building.'	
B2.3.2	B2.3.2 Individual building elements which are components of a building system and are difficult to access or replace must either: (a) All have the same durability, or (b) Be installed in a manner that permits the replacement of building elements of lesser durability without removing building elements that have greater durability and are not specifically designed for removal and replacement.	



Customer Service

Reid[™] Australia

Tel: 1300 780 250

Email: sales@itwcsanz.com
Web: www.reid.com.au

Reid™ New Zealand

Tel: 0800 88 22 12

Email: sales@ramsetreid.co.nz

Web: www.reids.co.nz

Reid™ Construction Systems (RCS) AUS: 1 Ramset Drive, Chirnside Park, Victoria, Australia, 3116 NZ: 23-29 Poland Road, Glenfield, Auckland 0632

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